

# Fold-and-thrust Belts

- ▶ Fold-and-thrust belts and submarine accretionary wedges share these features:
  - ▶ In cross-section, they occupy a **wedge-shaped deformed region** overlying a basal **detachment** or **décollement** fault.
  - ▶ The rocks or sediments beneath this fault show very little deformation.
  - ▶ The décollement fault characteristically dips toward the interior of the mountain belt or, in the case of a submarine wedge, toward the island arc
  - ▶ The topography, in contrast, slopes toward the toe or deformation front of the wedge.
  - ▶ Deformation within the wedge is generally dominated by imbricate thrust faults verging toward the toe and related fault-bend folding.

# Fold-and-thrust Belts

Examples of fold-and-thrust belts and accretionary wedges: Canadian Rocky Mountains and the Lesser Antilles

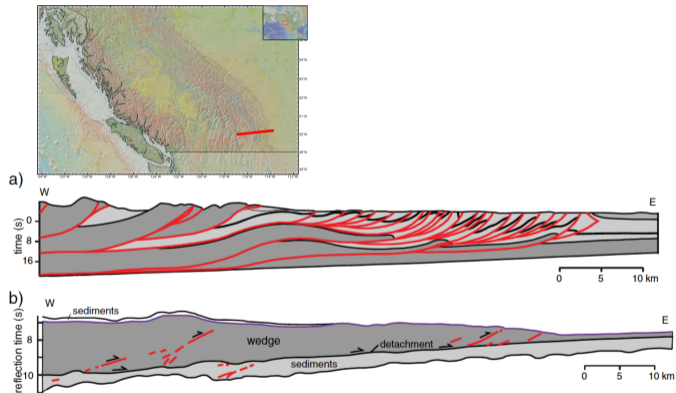
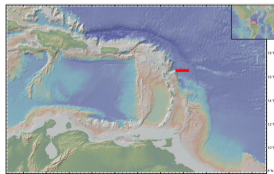


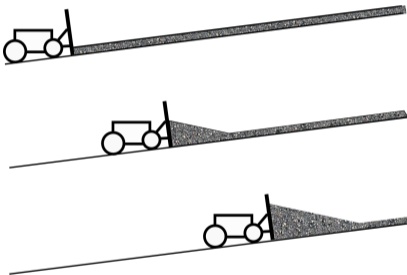
Fig. 1. a) Cross-section through the Canadian Rocky Mountains fold-and-thrust belt, redrawn from Bally et al. (1966). b) Cross-section through the Lesser Antilles accretionary wedge, redrawn from Westbrook et al. (1982).

(Buiter, Tectonophysics, 2012)



## Fold-and-thrust Belts

- ▶ The starting point of the mechanical theory for these structures is the recognition that they are analogous to a wedge of sand in front of a moving bulldozer.

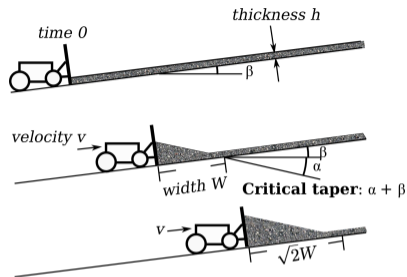


- ▶ The sand, rock, or sediment deforms until it develops a constant **critical taper**: i.e., the wedge slides stably without further deformation as it is pushed unless it encounters new fresh materials at the toe.

## Fold-and-thrust Belts

- ▶ The geometry of the critical taper is governed by the relative magnitude of the basal frictional resistance to internal strength.
- ▶ An increase in the basal resistance increases the critical taper while an increase in the wedge strength decreases the critical taper.
- ▶ The state of stress within a critically tapered wedge in the upper crust is everywhere on the verge of Coulomb failure since the taper is a product of continued brittle deformation.

# Mechanics of a Bulldozer Wedge: Kinematics



- ▶  $\alpha + \beta$ : the critical taper.
  - ▶  $\alpha$ : the surface slope of a deformed wedge
  - ▶  $\beta$ : the slope of a rigid hillside.
- ▶ From the mass conservation, we get the rate of the wedge growth for a constant density  $\rho$ :

$$\frac{d}{dt} \left[ \frac{1}{2} \rho W^2 \tan(\alpha + \beta) \right] = \rho h v. \quad (1)$$

# Mechanics of a Bulldozer Wedge: Kinematics

- ▶ By the definition of the critical taper,  $\alpha + \beta$  does not change in time. So, (1) becomes

$$W \frac{dW}{dt} = \frac{hv}{\tan(\alpha + \beta)}. \quad (2)$$

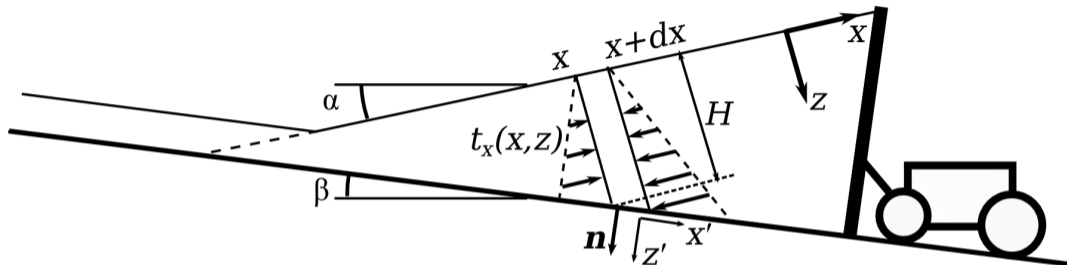
- ▶ The solution is

$$W = \left[ \frac{2hv}{\tan(\alpha + \beta)} \right]^{1/2} t^{1/2} \approx \left[ \frac{2hv}{\alpha + \beta} \right]^{1/2} t^{1/2}, \quad (3)$$

where the approximation is valid if  $\alpha + \beta \ll 1$  in radian.

- ▶ Since the coefficient is constant according to the assumptions we made, both the width and height(=  $W \tan(\alpha + \beta)$ ) grow like  $t^{1/2}$ .
- ▶ The growth is self-similar in the sense that the wedge at time  $2t$  is indistinguishable from the wedge at time  $t$ , magnified  $\sqrt{2}$  times.

## Mechanics of a Bulldozer Wedge: Critical Taper



- ▶ In the setting described above, we consider the force balance on an infinitesimal segment of the wedge lying between  $x$  and  $x + dx$ .
- ▶ First, a gravitational body force whose  $x$  component per unit length along strike is

$$F_g = -(\rho H dx) g \sin \alpha, \quad (4)$$

where  $g$  is the acceleration of gravity, and  $H$  is the local wedge thickness.

## Mechanics of a Bulldozer Wedge: Critical Taper

- ▶ Second, there is the net force exerted by the compressive tractions  $\sigma_{xx}$  acting on the sidewalls at  $x$  and  $x + dx$ . Setting compressive stress to be negative, we get this force as

$$\begin{aligned} F_s &= \int_0^H t_x(x) + t_x(x + dx) dz \\ &= \int_0^H (\boldsymbol{\sigma}(x) \cdot -\mathbf{e}_x)_x + (\boldsymbol{\sigma}(x + dx) \cdot \mathbf{e}_x)_x dz \\ &= \int_0^H [-\sigma_{xx}(x, z) + \sigma_{xx}(x + dx, z)] dz, \end{aligned} \tag{5}$$

where  $\mathbf{e}_x = (1, 0)$  is the unit basis vector for the  $x$  axis.



## Mechanics of a Bulldozer Wedge: Critical Taper

- ▶ Thirdly, and finally, there is the surface force exerted on the base.
- ▶ In a coordinate system of which  $x'$  axis is parallel to the bottom surface, we get the traction vector,  $\mathbf{t}' = \boldsymbol{\sigma}' \cdot (0, 1) = (\sigma_{x'z'}, \sigma_{z'z'}) \equiv (\tau_b, \sigma_n)$ .
- ▶ Transforming  $\mathbf{t}'$  to the  $x - z$  coordinate system,

$$\begin{bmatrix} t_x \\ t_z \end{bmatrix} = \begin{pmatrix} \cos(\alpha + \beta) & -\sin(\alpha + \beta) \\ \sin(\alpha + \beta) & \cos(\alpha + \beta) \end{pmatrix} \begin{bmatrix} \tau_b \\ \sigma_n \end{bmatrix}, \quad (6)$$

we get

$$F_b = t_x dx = [\tau_b \cos(\alpha + \beta) - \sigma_n \sin(\alpha + \beta)] dx. \quad (7)$$

- ▶ The base is governed by a frictional sliding condition,  $\tau_b = \mu_b(-\sigma_n)$ , where  $\mu_b$  is the basal friction coefficient. The sign is to make traction acting on  $+x$  direction when  $\sigma_n$  is compressive and thus negative. With this, we have

$$F_b = -\sigma_n [\mu_b \cos(\alpha + \beta) + \sin(\alpha + \beta)] dx. \quad (8)$$

# Mechanics of a Bulldozer Wedge: Critical Taper

- ▶ The force balance conditions is

$$F_g + F_s + F_b = 0. \quad (9)$$

- ▶ The first two forces,  $F_g$  and  $F_s$ , act in the  $-x$  direction, whereas  $F_b$  acts in the  $+x$  direction.
- ▶ We divide (9) with  $dx$  and assume  $dx \rightarrow 0$ . The result is

$$-\rho g H \sin \alpha - \sigma_n [\mu_b \cos(\alpha + \beta) + \sin(\alpha + \beta)] + \frac{d}{dx} \int_0^H \sigma_{xx} dz = 0. \quad (10)$$

- ▶ For  $\alpha \ll 1$  and  $\beta \ll 1$ , we employ the approximations  $\sin \alpha \approx \alpha$ ,  $\sin(\alpha + \beta) \approx \alpha + \beta$ ,  $\cos(\alpha + \beta) \approx 1$  and  $\sigma_n \approx -\rho g H$ . This reduces (10) to

$$\rho g H (\beta + \mu_b) + \frac{d}{dx} \int_0^H \sigma_{xx} dz \approx 0. \quad (11)$$

# Mechanics of a Bulldozer Wedge: Critical Taper

- ▶ The failure criterion for non-cohesive dry sand can be written in the form

$$\frac{\sigma_1}{\sigma_3} = \frac{1 + \sin \phi}{1 - \sin \phi}, \quad (12)$$

where  $\sigma_1$  and  $\sigma_3$  are the greatest and least principal compressive stresses, respectively, and  $\phi$  is the internal friction angle.

- ▶ From the last class, we know that frictional sliding satisfies  $\sigma_s = \tan \phi \sigma_n$  (cohesion is zero since non-cohesive!) and also

$$\sigma_s = \frac{\sigma_1 - \sigma_3}{2} \sin 2\theta = \frac{\sigma_1 - \sigma_3}{2} \cos \phi, \quad (13)$$

$$\sigma_n = \frac{\sigma_1 + \sigma_3}{2} - \frac{\sigma_1 - \sigma_3}{2} \sin 2\theta = \frac{\sigma_1 + \sigma_3}{2} - \frac{\sigma_1 - \sigma_3}{2} \cos \phi, \quad (14)$$

since  $2\theta = \pi/2 - \phi$  under the failure condition.

- ▶ Plugging (13) and (14) into the sliding condition, we get (12).

# Mechanics of a Bulldozer Wedge: Critical Taper

- ▶ In a narrow taper (i.e.,  $\alpha \ll 1$  and  $\beta \ll 1$ ),  $\sigma_1$  and  $\sigma_3$  are approximately horizontal and vertical:

$$\sigma_{xx} \approx \sigma_1 \approx -\frac{1 + \sin \phi}{1 - \sin \phi} \rho g z, \quad (15)$$

$$\sigma_{zz} \approx \sigma_3 \approx -\rho g z. \quad (16)$$

- ▶ The traction acting on the sidewalls then reduces in this approximation to

$$\frac{d}{dx} \int_0^H \sigma_{xx} dz \approx -\frac{1 + \sin \phi}{1 - \sin \phi} \rho g H (\alpha + \beta), \quad (17)$$

where we have used the relation  $dH/dx = \tan(\alpha + \beta) \approx \alpha + \beta$ .

# Mechanics of a Bulldozer Wedge: Critical Taper

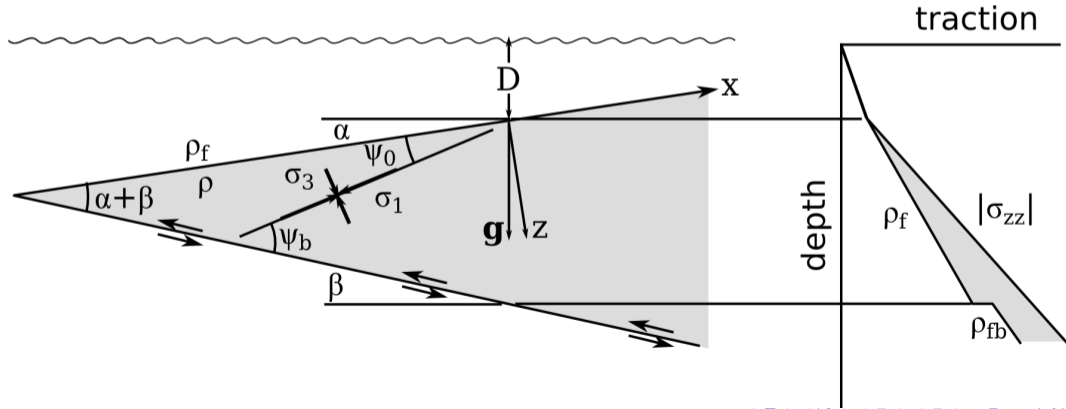
- ▶ Substituting (17) into (11), we obtain the approximate critical taper equation for a dry sand wedge in front of a bulldozer:

$$\alpha + \beta \approx \frac{1 + \sin \phi}{1 - \sin \phi} (\beta + \mu_b). \quad (18)$$

- ▶ The critical taper,  $\alpha + \beta$  is proportional to the basal friction coefficient,  $\mu_b$ ; inversely to the internal friction angle,  $\phi$ .

## Noncohesive Coulomb Wedge

- ▶ Now, we want to consider the effects of pore fluid pressure on the stability of the wedge.
- ▶ Also, we want to get exact solutions.
- ▶ Here is the problem setting:



## Noncohesive Coulomb Wedge

- ▶ The static equilibrium equation is

$$\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xz}}{\partial z} - \rho g z \sin \alpha = 0, \quad (19)$$

$$\frac{\partial \sigma_{xz}}{\partial x} + \frac{\partial \sigma_{zz}}{\partial z} + \rho g z \cos \alpha = 0. \quad (20)$$

- ▶ Boundary conditions on the upper surface of the wedge:

$$\sigma_{xz} = 0, \quad \sigma_{zz} = -\rho_f g D, \quad (21)$$

where  $D$  is the water depth.

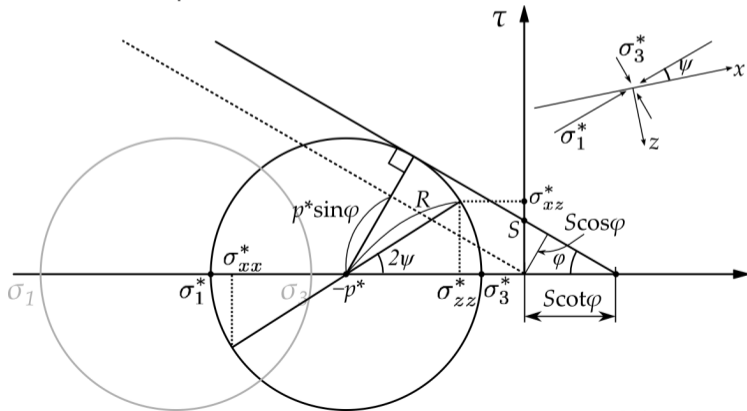
- ▶ It is convenient to define the generalized Hubbert-Rubey pore fluid to lithostatic pressure ratio by

$$\lambda = \frac{\rho_f - \rho_f g D}{-\sigma_{zz} - \rho_f g D}. \quad (22)$$

- ▶ We assume that  $\lambda$ ,  $\rho$  and the internal friction coefficient ( $\mu$ ) are all constant.

## Noncohesive Coulomb Wedge

- ▶ The density  $\rho$  is actually defined as  $\rho = (1 - \eta)\rho_s + \eta\rho_f$ . Since the rock ( $\rho_s$ ) and fluid density ( $\rho_f$ ) are constant, the constant aggregate density (i.e. constant  $\rho$ ) implies a constant porosity,  $\eta$ .
- ▶ For convenience, we need to work out some more relations between stress components and other parameters.





# Noncohesive Coulomb Wedge

- ▶ From geometry,

$$p^* = -\frac{1}{2}(\sigma_{zz}^* + \sigma_{xx}^*), \quad (23)$$

$$R = p^* \sin \phi + S \cos \phi, \quad (24)$$

$$\sigma_{xx}^* = -p^* - R \cos 2\psi, \quad (25)$$

$$\sigma_{zz}^* = -p^* + R \cos 2\psi, \quad (26)$$

$$\sigma_{xz}^* = R \sin 2\psi. \quad (27)$$

- ▶ From these relations, we derive the following expressions for later uses:

$$\begin{aligned} \frac{\sigma_{zz}^* - \sigma_{xx}^*}{2} &= -\frac{\sigma_{zz}^* + \sigma_{xx}^*}{2} \sin \phi \cos 2\psi + S \cos \phi \cos 2\psi \\ \Rightarrow \frac{\sigma_{zz}^* - \sigma_{xx}^*}{2} &= \frac{S \cot \phi - \sigma_{zz}^*}{\csc \phi \sec 2\psi - 1}. \end{aligned} \quad (28)$$

$$\sigma_{xz}^* = \tan 2\psi \frac{S \cot \phi - \sigma_{zz}^*}{\csc \phi \sec 2\psi - 1}. \quad (29)$$

# Noncohesive Coulomb Wedge

- ▶ The following stress components satisfy the static momentum balance, the failure condition and the boundary conditions on the top surface:

$$\sigma_{xz}^* = (\rho - \rho_f)gz \sin \alpha, \quad (30)$$

$$\sigma_{zz}^* = -\rho_f gD - \rho gz \cos \alpha, \quad (31)$$

$$\sigma_{xx}^* = -\rho_f gD - \rho gz \cos \alpha \frac{\csc \phi \sec 2\psi_0 - 2\lambda + 1}{\csc \phi \sec 2\psi_0 - 1}, \quad (32)$$

provided that

$$\frac{\tan 2\psi_0}{\csc \phi \sec 2\psi_0 - 1} = \left( \frac{1 - \rho_f/\rho}{1 - \lambda} \right) \tan \alpha. \quad (33)$$

- ▶ (33) relates the stress orientation angle  $\psi_0$  to the surface slope  $\alpha$ ; we have assumed that  $\psi_0$  is constant and have made use of the relation  $dD/dx = -\sin \alpha$ .

## Noncohesive Coulomb Wedge

- ▶ (30), (31), (32) are an exact solution for the state of stress in a sloping half-space on the verge of Coulomb failure.
- ▶ All that remains is to satisfy the basal boundary condition. Allowing for a different pore-fluid regime (like a different porosity) on the décollement fault, we have this boundary condition for the base:

$$\tau_b = -\mu_b(\sigma_n + p_{fb}), \quad (34)$$

where  $p_{fb}$  is the pore-fluid pressure on the base and  $\mu_b$  is the basal coefficient of friction.

- ▶ Expressing the basal shear and normal stress in terms of stress components in the  $x - z$  coordinate system:

$$\tau_b = 1/2(\sigma_{zz}^* - \sigma_{xx}^*) \sin 2(\alpha + \beta) + \sigma_{xz}^* \cos 2(\alpha + \beta), \quad (35)$$

$$\sigma_n = \sigma_{zz}^* - \sigma_{xz}^* \sin 2(\alpha + \beta) - 1/2(\sigma_{zz}^* - \sigma_{xx}^*)[1 - \cos 2(\alpha + \beta)]. \quad (36)$$

## Noncohesive Coulomb Wedge

- ▶ (30), (31), (32), (35) and (36) are used to determine the dip of the surface on which the frictional sliding condition (34) is satisfied.
- ▶ After some algebra,  $\beta$  is given by

$$\alpha + \beta = \psi_b - \psi_0, \quad (37)$$

where

$$\frac{\tan 2\psi_b}{\csc \phi \sec 2\psi_b - 1} = \mu_b \left( \frac{1 - \lambda_b}{1 - \lambda} \right). \quad (38)$$

- ▶ (37) is the exact critical taper equation for a homogeneous noncohesive Coulomb wedge.

# Noncohesive Coulomb Wedge

- ▶ Here is the usual procedure we take to play with the above critical taper equation.

1. We can start from (38) to find  $\psi_b$  from  $\mu_b$  and  $\lambda_b$ .

$$\frac{\tan 2\psi_b}{\csc \phi \sec 2\psi_b - 1} = \mu_b \left( \frac{1 - \lambda_b}{1 - \lambda} \right).$$

2. Then, assume a value of the surface slope,  $\alpha$ , to get  $\psi_0$  from (33).

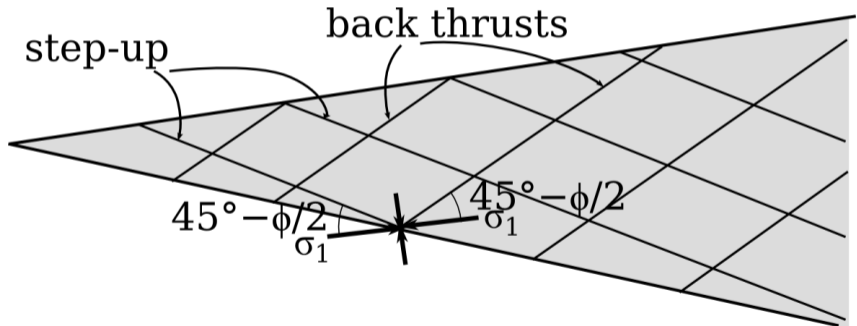
$$\frac{\tan 2\psi_0}{\csc \phi \sec 2\psi_0 - 1} = \left( \frac{1 - \rho_f/\rho}{1 - \lambda} \right) \tan \alpha.$$

3. Since  $\psi_b$ ,  $\psi_0$  and  $\alpha$  are known, we get  $\beta$  from the taper equation, (37).

$$\beta = \psi_b - \psi_0 - \alpha.$$

## Noncohesive Coulomb Wedge

- ▶ Once we get all these angles, we can predict the orientation of step-up from the basal décollement fault as well as that of backthrust faults.



# Noncohesive Coul

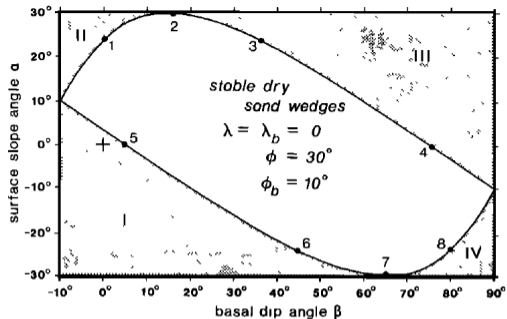
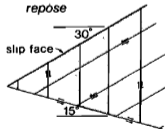


Fig. 9. Diagram showing stable and unstable regions of dry sand wedges having  $\phi = 30^\circ$  and  $\phi_b = 10^\circ$ . Critical wedges labeled 1–8 are depicted in Figure 10.

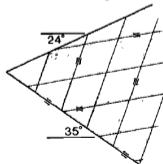
1. normal faulting and downslope flow



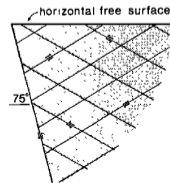
2. surface at angle of repose



3. combined normal and thrust faulting



4. thrust faulting



5. accretionary wedge fails

6. combined normal and

7. surface at angle